# **CHAPTER 08: PIECE-WISE LINEAR FINITE ELEMENT SPACE**

## **GLOBAL COORDINATE SYSTEM**

Let's review piece-wise linear shape function. A sketch of the shape function is shown below.



The shape function is defined as follows:

$$
A = 1
$$
\n
$$
N_{A}(x) = \begin{cases} \frac{x_{2} - x}{h_{1}} & \text{for } x_{1} \leq x \leq x_{2} \\ 0 & \text{elsewhere} \end{cases}
$$
\n
$$
2 \leq A \leq n
$$
\n
$$
N_{A}(x) = \begin{cases} \frac{x - x_{A-1}}{h_{A-1}} & \text{for } x_{A-1} \leq x \leq x_{A} \\ \frac{x_{A+1} - x}{h_{A}} & \text{for } x_{A} \leq x \leq x_{A+1} \end{cases}
$$
\n
$$
A = n + 1
$$
\n
$$
N_{A}(x) = \begin{cases} \frac{x - x_{n}}{h_{n}} & \text{for } x_{n} \leq x \leq x_{n+1} \\ 0 & \text{elsewhere} \end{cases}
$$
\n
$$
A = n + 1
$$
\n
$$
N_{A}(x) = \begin{cases} \frac{x - x_{n}}{h_{n}} & \text{for } x_{n} \leq x \leq x_{n+1} \\ 0 & \text{elsewhere} \end{cases}
$$

You may have recognized that the above equation for  $N_A(x)$ only shows the slope. (The intercept at  $x_1$  is omitted.) In fact, it is ok because later we are only focusing on the stiffness matrix which is calculated from the derivative of the slope of the shape function (and thus, any constants will drop form the equation).

Here, "h" is a mesh size. That is, in general,

hA = x <sup>A</sup>+<sup>1</sup> − x <sup>A</sup> ..Eq.08-1

The subdomain on h<sub>A</sub> is called finite element space, or simply element.

### **ELEMENT POINT OF VIEW**

So far we have been talking everything in global space (or global coordinate system). Now let's introduce **Element Space** (or element coordinate system). Element space is also called **local coordinate system**.

Each element domain is defined using  $\xi_1$  and  $\xi_2$ , and have value -1 and 1, respectively. The following shows comparison of Global and Element spaces.



#### **SHAPE FUNCTION IN ELEMENT SPACE**

The shape function in terms of local coordinate system (in terms of  $\xi$ ) can be shown as follows:

$$
N_1(\xi) = -\frac{1}{2}\xi + \frac{1}{2}
$$
  

$$
N_2(\xi) = \frac{1}{2}\xi + \frac{1}{2}
$$

Or, the above two equations can be expressed in one equation as follows:

$$
N_a(\xi) = \frac{1}{2} (1 + \xi_a \xi) \qquad a = 1, 2 \dots
$$
Eq.08-2  

$$
\xi_1 = -1 \text{ and } \xi_2 = 1
$$

## **LOCAL AND GLOBAL LOCATION**

The local location in terms of global variable (that is,  $\xi(x)$ ) can be expressed, using a constants  $c_1$  and  $c_2$ , as,

$$
\xi(\mathbf{x}) = \mathbf{c}_1 + \mathbf{c}_2 \cdot \mathbf{x}
$$

The constant  $c_1$  and  $c_2$  can be determined by

x(x <sup>A</sup> ) = c <sup>1</sup> + c <sup>2</sup> ⋅ x <sup>A</sup> = −1 ...Eq.08-3 x(x <sup>A</sup>+<sup>1</sup> ) = c <sup>1</sup> + c <sup>2</sup> ⋅ x <sup>A</sup>+<sup>1</sup> = 1 ...Eq.08-4

Solving for  $c_1$  from Eq.08-3 and inserting it into Eq.08-4, we get

$$
(-1 - c_2 \cdot x_A) + c_2 \cdot x_{A+1} = 1
$$
  
\n
$$
\Rightarrow c_2 \cdot (x_{A+1} - x_A) = 2
$$
  
\n
$$
\Rightarrow c_2 = \frac{2}{x_{A+1} - x_A}
$$

Therefore, inserting this  $c_2$  into Eq.08-3,

$$
c_{1} + \frac{2}{x_{A+1} - x_{A}} \cdot x_{A} = -1
$$
\n
$$
\Rightarrow c_{1} = -\frac{2}{x_{A+1} - x_{A}} \cdot x_{A} - 1 = -\frac{2x_{A}}{x_{A+1} - x_{A}} - \frac{x_{A+1} - x_{A}}{x_{A+1} - x_{A}} = -\left(\frac{2x_{A}}{x_{A+1} - x_{A}} + \frac{x_{A+1} - x_{A}}{x_{A+1} - x_{A}}\right) = -\frac{x_{A} + x_{A+1}}{x_{A+1} - x_{A}}
$$

Therefore, inserting  $c_1$  and  $c_2$ ,

$$
\xi(x) = -\frac{x_A + x_{A+1}}{x_{A+1} - x_A} + \frac{2}{x_{A+1} - x_A} \cdot x
$$

$$
\Rightarrow \xi(x) = \frac{2x - x_A - x_{A+1}}{x_{A+1} - x_A}
$$

Here, since  $h_A = x_{A+1} - x_A$  (Eq.08-1), we get

( ) A A A 1 h 2x x x <sup>x</sup> <sup>−</sup> <sup>−</sup> <sup>+</sup> <sup>x</sup> <sup>=</sup> ..Eq.08-5

If we solving Eq.08-5 for x, we can obtain global location in terms of local variable, that is,  $x(\xi)$ .

$$
\Rightarrow h_A \xi = 2x - x_A - x_{A+1}
$$

Therefore,

( ) <sup>2</sup> h x x <sup>x</sup> <sup>A</sup> <sup>+</sup> <sup>A</sup> <sup>+</sup> <sup>A</sup>+<sup>1</sup> <sup>x</sup> <sup>x</sup> <sup>=</sup> ..Eq.08-6

In local coordinate system,  $h_A = x_{A+1} - x_A$ , the Eq.08-6 can be written as

$$
x(\xi) = \frac{(x_{A+1} - x_A)\xi + x_A + x_{A+1}}{2}
$$

Further, using  $x_A = x_1^e$ , and  $x_{A+1} = x_2^e$ , we can express it as

$$
x(\xi) = x^e(\xi) = \frac{(x_2^e - x_1^e)\xi + x_1^e + x_2^e}{2} = \frac{1}{2}(1 - \xi)x_1^e + \frac{1}{2}(1 + \xi)x_2^e
$$

Remember, from Eq.08-2, we have

$$
N_a\left(\xi\right)=\frac{1}{2}\big(1+\xi_a\xi\big)\hspace{1cm}a=1,\,2
$$

Therefore, the above  $x^e(\xi)$  can be shown as

$$
x^{e}(\xi) = \underbrace{\frac{1}{2}(1-\xi)x_{1}^{e}}_{N_{1}(\xi)} + \underbrace{\frac{1}{2}(1+\xi)x_{2}^{e}}_{N_{2}(\xi)} = N_{1}x_{1}^{e} + N_{2}x_{2}^{e}
$$

Finally, in a simple form, the above equation can be written as

$$
x^e(\xi) = \sum_{a=1}^2 N_a(\xi) x^e_a
$$

## **OTHER IMPORTANT EQUATIONS FOR FUTURE REFERENCES**

Let's find some other important relations for future purpose.

Taking the derivative of Eq.08-2 with respect to ξ ,

$$
\frac{d}{d\xi}N_a=\frac{d}{d\xi}\bigg[\frac{1}{2}\big(1+\xi_a\xi\big)\bigg]=\frac{\xi_a}{2}=\frac{\big(-1\big)^a}{2}
$$

Thus,

( ) 2 <sup>1</sup> <sup>N</sup> d d <sup>a</sup> a <sup>−</sup> <sup>=</sup> <sup>ξ</sup> ...Eq.08-8

Taking the derivative of Eq.08-6 with respect to ξ ,

$$
\frac{d}{d\xi}x^e=\frac{d}{d\xi}\Bigg(\frac{h_{A}\xi+x_{A}+x_{A+1}}{2}\Bigg)=\frac{d}{d\xi}\Bigg(\frac{h^e\xi+x_1^e+x_2^e}{2}\Bigg)=\frac{h^e}{2}
$$

Thus,

2 h x d d <sup>e</sup> <sup>e</sup> <sup>=</sup> <sup>x</sup> ..Eq.08-9

Taking the derivative of Eq.08-5 with respect to x,

$$
\frac{d}{dx}\,\xi=\frac{d}{dx}\Bigg(\frac{2x-x_{\text{A}}-x_{\text{A+1}}}{h_{\text{A}}}\Bigg)
$$

In terms of element space  $\xi^e$ ,

$$
\frac{d}{dx}\xi^e=\frac{d}{dx}\Bigg(\frac{2x-x_1^e-x_2^e}{h^e}\Bigg)=\frac{2}{h^e}
$$

Thus,

e e h 2 dx <sup>d</sup> <sup>x</sup> <sup>=</sup> ..Eq.08-10

From Eq.08-9 and Eq.08-10, we also can say,

$$
\frac{d}{dx}\xi^{e} = \left(\frac{d}{d\xi}x^{e}\right)^{-1}
$$

### **ASSEMBLING GLOBAL MATRIX FORMS**

Let's review a global matrix form for the following general nodes and elements.



The force vectors and stiffness matrix in a global matrix form (Eq.05-10) has the following sizes.

 ${F_A} = [K_{AB}](d_B)$ n x 1 n x n

The global integral can be shown as a sum of local integral over the element domain. Therefore, stiffness matrix and force vectors can be shown as,

 $[K_{AB}]$  =  $\sum_{e=1}^{n_{el}} [k_{ab}^e]$  $[K_{AB}]$  =  $\sum_{e=1}^{n}$   $k_{ab}^e$  $\left\{ \textbf{F}_{\textbf{A}}^{\phantom{\dag}}\right\} =\sum_{\textbf{e}=1}^{n_{\textbf{e}l}}\left\{ \textbf{f}_{\textbf{a}}^{\phantom{\dag}}\right\}$ e=1  $\left\{ \mathsf{F}_{_{\mathsf{A}}}\right\} =\sum\limits_{\mathsf{A}}\mathsf{f}_{_{\mathsf{a}}}^{^{\mathsf{e}}}\mathsf{f}_{_{\mathsf{a}}}^{^{\mathsf{e}}}$ ,where  $\begin{bmatrix} \mathsf{k}_{\mathsf{a}\mathsf{b}}^{\mathsf{e}} \end{bmatrix} = \int\limits_{\Omega^{\mathsf{i}}}$ = e N dx dx <sup>d</sup> <sup>N</sup> dx <sup>d</sup> <sup>k</sup> <sup>a</sup> <sup>b</sup> e ab ...Eq.08-12  $\left\langle \mathbf{f}_{\mathbf{a}}^{\mathbf{e}}\right\rangle =\int_{\Omega^{\mathbf{e}}}N_{\mathbf{a}}f\mathbf{dx}+$  $\textsf{f}^\texttt{e}_\textsf{a} \big\}$ =  $\big\{\textsf{N}_\textsf{a}\textsf{f}$ dx  $\delta_{\mathsf{a}1}h$  for e = 1 0 for  $e = 2, 3, ..., n_{el} - 1$  $-\delta$ <sub>a2</sub> $g$  for e = n<sub>el</sub>  $\Omega^e = [x_1^e, x_2^e]$  (The domain of the element) Size of local matrix and vector  $= 2 \times 2$  $\{ \}$ e $\}$  $f^e$  = 2 x 1

The following figure depicts how in general the local components are assembled into global matrix.



### **CHANGE INTEGRAL FROM GLOBAL TO ELEMENTAL COORDINATE SYSTEM**

Now, let's review two general mathematical formulas.

First, the **Change of Variable Formula**, which we want to convert x (global coordinate) in terms of ξ (local coordinate), is given in the following formula:

$$
\int_{x_1}^{x_2}f\big(x\big)dx=\int_{\xi_1}^{\xi_2}f\big(x\big(\xi\big)\big)\frac{d}{d\xi}\,x\big(\xi\big)d\xi
$$

By the way, in Change of Variable Formula, the term  $\frac{d}{d\xi}x(\xi)$  is known as **Jacobian Determinant**. We'll talk about this later.

Second, the **Chain Rule** is given in the following formula:

$$
\frac{\partial}{\partial \xi} f(x(\xi)) = \frac{\partial}{\partial x} f(x(\xi)) \frac{\partial}{\partial \xi} x(\xi)
$$

By knowing the above two formulas, now let's convert the stiffness matrix from global coordinate system to a local, element coordinate system.

We start from Eq.08-12 as

$$
\left[k_{ab}^e\right]=\int\limits_{\Omega^e} \frac{d}{dx}N_a\,\frac{d}{dx}N_b dx \hspace{1.5cm} (Eq.08-12)
$$

By using a change of variable formula, it becomes

$$
\Rightarrow \left[k_{ab}^e\right] = \int\limits_{-1}^{+1} \frac{d}{dx} N_a\big(x(\xi)\big) \frac{d}{dx} N_b\big(x(\xi)\big) \frac{d}{d\xi} \, x\big(\xi\big) d\xi
$$

Then, by using chain rule,

$$
\Rightarrow [k_{ab}^e] = \iint_{-1}^{+1} \frac{\frac{d}{d\xi} N_a(\xi)}{\frac{d}{d\xi} x(\xi)} \left[ \frac{\frac{d}{d x} N_b(\xi)}{\frac{d}{d\xi} x(\xi)} \right] \frac{d}{d\xi} x(\xi) d\xi
$$

$$
= \int_{-1}^{+1} \frac{d}{d\xi} N_a(\xi) \left[ \frac{d}{d\xi} x(\xi) \right]^{-1} \frac{d}{d\xi} N_b(\xi) \left[ \frac{d}{d\xi} x(\xi) \right]^{-1} \frac{d}{d\xi} x(\xi) d\xi
$$

cancelled out

$$
=\int\limits_{-1}^{+1}\!\frac{d}{d\xi}N_a\Big(\xi\Big)\frac{d}{d\xi}N_b\Big(\xi\!\Bigg[\frac{d}{d\xi}x\Big(\xi\Big)\!\Bigg]^{-1}\!d\xi
$$

Using change of variable formula,  
\n
$$
\int_{\Omega^e} (\qquad) dx \Rightarrow \int_{-1}^{+1} (\qquad) \frac{d}{d\xi} x(\xi) d\xi
$$

Using chain rule,  
\n
$$
\frac{d}{d\xi} N(x(\xi)) = \frac{d}{dx} N(x(\xi)) \frac{d}{d\xi} x(\xi)
$$
\nTherefore,  
\n
$$
\frac{d}{dx} N(x(\xi)) = \frac{d}{dx} N(\xi) = \frac{\frac{d}{d\xi} N(x(\xi))}{\frac{d}{d\xi} x(\xi)}
$$

Now, recall the following equations from previous pages.

$$
\frac{d}{d\xi}N_a = \frac{(-1)^a}{2}
$$
 (Eq.08-8)  

$$
\frac{d}{d\xi}x^e = \frac{h^e}{2}
$$
 (Eq.08-9.)

Substituting these equations, then, the stiffness matrix equation becomes,

$$
\left[k_{ab}^{e}\right]=\int\limits_{-1}^{+1}\frac{d}{d\xi}N_{a}(\xi)\frac{d}{d\xi}N_{b}(\xi)\Bigg[\frac{d}{d\xi}x(\xi)\Bigg]^{-1}d\xi=\int\limits_{-1}^{+1}\frac{(-1)^{a}}{2}\cdot\frac{(-1)^{b}}{2}\cdot\frac{2}{h^{e}}d\xi=\int\limits_{-1}^{+1}\frac{(-1)^{a+b}}{2h^{e}}d\xi=\frac{(-1)^{a+b}}{2h^{e}}\xi\Bigg|_{-1}^{+1}=\frac{(-1)^{a+b}}{h^{e}}
$$

Therefore, each component of  $\left[\mathsf{k}_{\mathsf{ab}}^{\mathsf{e}}\right]$  is,

$$
k_{11}^e = \frac{(-1)^{1+1}}{h^e} = \frac{1}{h^e}
$$

$$
k_{12}^{e} = \frac{(-1)^{1+2}}{h^{e}} = -\frac{1}{h^{e}}
$$

$$
k_{21}^{e} = \frac{(-1)^{2+1}}{h^{e}} = -\frac{1}{h^{e}}
$$

$$
k_{22}^{e} = \frac{(-1)^{2+2}}{h^{e}} = \frac{1}{h^{e}}
$$

Therefore, we can express  $\left| {\sf k}^{\scriptscriptstyle\rm e}_{\scriptscriptstyle\rm ab}\right|$  in a very simple form as follows:

[ ] − <sup>−</sup> <sup>=</sup> <sup>1</sup> <sup>1</sup> 1 1 h <sup>1</sup> <sup>k</sup> <sup>e</sup> e ab ...Eq.08-13

## **EXAMPLE**

Let's do an example problem of using local stiffness matrix and assembling into a global stiffness matrix. We'll be using the same problem as we did in Chapter 07.

In Eq.07-1, we had the following problem,

$$
u(t) = 0
$$
\n
$$
u(t) = 0
$$
\n
$$
u(t) = 0
$$
\n
$$
-\frac{d}{dx}u(0) = 0
$$

And, we approximated at the following points.

$$
x=0\;,\;\frac{1}{3}\;,\;\frac{2}{3}\;,\;1
$$



The global stiffness matrix was,

$$
[K_{AB}] = \begin{bmatrix} 3 & -3 & 0 & 0 \\ -3 & 6 & -3 & 0 \\ 0 & -3 & 6 & -3 \\ 0 & 0 & -3 & 3 \end{bmatrix}
$$
 (Eq.07-4)

Now, let's look at the element space.

The number of element is,

$$
n_{\text{el}}=3\,
$$

Mesh size of each elements is all equal to  $\frac{1}{3}$ , that is,

$$
h^e = h_1 = h_2 = h_3 = \frac{1}{3}
$$

Therefore, each local stiffness matrix is all equal to,

$$
\[mathbf{k}^{\mathbf{e}}\] = \begin{bmatrix} \mathbf{k}_1 \end{bmatrix} = \begin{bmatrix} \mathbf{k}_2 \end{bmatrix} = \begin{bmatrix} \mathbf{k}_3 \end{bmatrix} = \frac{1}{\mathbf{h}^{\mathbf{e}}} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} = \frac{1}{\left(\frac{1}{3}\right)} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} = \begin{bmatrix} 3 & -3 \\ -3 & 3 \end{bmatrix}
$$

The global stiffness matrix is an assembly of these local stiffness matrixes as the following figure.



Thus, we had the same global stiffness matrix as Eq.07-4; however, it was much easily obtained without doing any integral calculations this time.